Longitudinal strength standard

S11.1 Application

This requirement applies only to steel ships of length 90 m and greater in unrestricted service. For ships having one or more of the following characteristics, special additional considerations will be given by each Classification Society.

(i) Proportion $L/B \leq 5$, $B/D \geq 2.5$
(ii) Length $L \geq 500$ m
(iii) Block coefficient $Cb < 0.6$
(iv) Large deck opening
(v) Ships with large flare
(vi) Carriage of heated cargoes
(vii) Unusual type or design

For bulk carriers with notation BC-A, BC-B or BC-C, as defined in UR S25, this UR is to be complied with by ships contracted for construction on or after 1 July 2003. For other ships, this revision of this UR is to be complied with by ships contracted for construction on or after 1 July 2004.

S11.2 Loads

S11.2.1 Still water bending moment and shear force

S11.2.1.1 General

Still water bending moments, $Ms$ (kN-m), and still water shear forces, $Fs$ (kN), are to be calculated at each section along the ship length for design cargo and ballast loading conditions and ballast conditions as specified in S11.2.1.2.

For these calculations, downward loads are assumed to be taken as positive values, and are to be integrated in the forward direction from the aft end of $L$. The sign conventions of $Ms$ and $Fs$ are as shown in Fig. 1.

![Sign Conventions of MS and Fs](image)

S11-1
General cargo ships, container ships, roll-on/roll-off and refrigerated cargo carriers, bulk carriers, ore carriers:
- Homogeneous loading conditions at maximum draught
- Ballast conditions
- Special loading conditions e.g., container or light load conditions at less than the maximum draught, heavy cargo, empty holds or non-homogeneous cargo conditions, deck cargo conditions, etc., where applicable.
- All loading conditions specified in UR S25 Section 4 for bulk carriers with notation BC-A, BC-B or BC-C, as applicable.

Oil tankers:
- Homogeneous loading conditions (excluding dry and clean ballast tanks) and ballast or part loaded conditions
- Any specified non-uniform distribution of loading
- Mid-voyage conditions relating to tank cleaning or other operations where these differ significantly from the ballast conditions.

Chemical tankers:
- Conditions as specified for oil tankers
- Conditions for high density or segregated cargo.

Liquefied gas carriers:
- Homogeneous loading conditions for all approved cargoes
- Ballast conditions
- Cargo conditions where one or more tanks are empty or partially filled or where more than one type of cargo having significantly different densities are carried.

Combination Carriers:
- Conditions as specified for oil tankers and cargo ships.

S11.2.1.3 Partially filled ballast tanks in ballast loading conditions

Ballast loading conditions involving partially filled peak and/or other ballast tanks at departure, arrival or during intermediate conditions are not permitted to be used as design conditions unless; where alternative filling levels would result in design stress limits being exceeded. The partial filling of such tanks is, however, permitted in service to satisfy operational requirements providing:

- design stress limits are satisfied for all intermediate filling levels between empty and full and
- for bulk carriers, UR S17, as applicable, is complied with for all filling levels between empty and full.

However, for the purpose of design, it will be acceptable if, in each condition at departure, arrival and, where required by S11.2.1.2, any intermediate condition, the tanks intended to be partially filled are assumed to be empty and full. In addition, the specified partly filled level in the intended condition is to be considered.

S11.2.1.4 Partially filled ballast tanks in cargo loading conditions

In cargo loading conditions, the requirement in S11.2.1.3 applies to the peak tanks only.

S11.2.2 Wave loads

S11.2.2.1 Wave bending moment

The wave bending moments, $M_w$, at each section along the ship length are given by the following formulae:

$$M_w (+) = + 190 MC L^2 B C_b x 10^{-3} \text{ (kN} \cdot \text{m) ... For positive moment}$$

$$M_w (+) = - 110 MC L^2 B (C_b + 0.7) x 10^{-3} \text{ (kN} \cdot \text{m) ... For negative moment}$$

where, $M =$ Distribution factor given in Fig. 2

$$C = 10.75 - \left[ \frac{300 - L}{100} \right]^{15} \text{ for } 90 \leq L \leq 300$$

or $10.75,$ for $300 < L < 350$

or $10.75 - \left[ \frac{L - 350}{150} \right]^{15} \text{ for } 350 \leq L \leq 500$

$L =$ Length of the ships in metres, defined by S2

$B =$ Greatest moulded breadth in metres

$C_b =$ Block coefficient, defined by S2, but not to be taken less than 0.6
S11 2.2.2 Wave shear force

The wave shear forces, $F_w$, at each section along the length of the ship are given by the following formulae:

$$F_w^+ = + 30 F_1 C L B (C_b + 0.7) \times 10^{-2} \text{ (kN)}$$  ... For positive shear force

$$F_w^- = - 30 F_2 C L B (C_b + 0.7) \times 10^{-2} \text{ (kN)}$$  ... For negative shear force

Where,  $F_1, F_2$ = Distribution factors given in Figs. 3 and 4

$C, L, B, C_b$ = As specified in S11.2.2.1

---

**Fig. 2 Distribution factor $M$**

**Fig. 3 Distribution factor $F_1$**
S11.3 Bending strength

S11.3.1 Bending strength amidships

S11.3.1.1 Section modulus

(i) Hull section modulus, Z, calculated in accordance with S5, is not to be less than the values given by the following formula in way of 0.4 L amidships for the still water bending moments $M_s$ given in S11.2.1.1 and the wave bending moments $M_w$ given in S11.2.2.1, respectively:

$$\frac{|M_s + M_w|}{\sigma} \times 10^3 \ (cm^3)$$

where, $\sigma =$ permissible bending stress = 175 / $k$ (N/mm$^2$)

$k = 1.0$ for ordinary hull structural steel

$k < 1.0$ for higher tensile steel according to S4.

(ii) In any case, the longitudinal strength of the ship is to be in compliance with S7.

S11.3.2 Moment of inertia

Moment of inertia of hull section at the midship point is not to be less than

$$I_{min} = 3CL^3B \ (Cb + 0.7) \ (cm^4)$$

Where $C$, $L$, $B$, $Cb$ = As specified in S11.2.2.1.

S11.3.2 Bending strength outside amidships.

The required bending strength outside 0.4 L amidships is to be determined at the discretion of each Classification Society.

S11.4 Shearing strength

S11.4.1 General

The thickness requirements given in S11.4.2 or S11.4.3 apply unless smaller values are proved satisfactory by a method of direct stress calculation approved by each Classification Society, where the
calculated shear stress is not to exceed 110/k (N/mm²).

**S11.4.2 Shearing strength for ships without effective longitudinal bulkheads**

(i) The thickness of side shell is not to be less than the values given by the following formula for the still water shear forces $F_s$ given in S11.2.1.1 and the wave shear forces $F_w$ given in S11.2.2.2, respectively:

\[
t = \frac{0.5 \left[ F_s + F_w \right]}{\tau} \frac{S}{I} \times 10^2 \quad \text{(mm)}
\]

where, $I$ = Moment of inertia in cm⁴ about the horizontal neutral axis at the section under consideration
S = First moment in cm³, about the neutral axis, of the area of the effective longitudinal members between the vertical level at which the shear stress is being determined and the vertical extremity of effective longitudinal members, taken at the section under consideration
\(\tau\) = permissible shear stress = 110/k (N/mm²)
\(k\) = As specified in S11.3.1.1 (i)

(ii) The value of $F_s$ may be corrected for the direct transmission of forces to the transverse bulkheads at the discretion of each Classification Society.

**S11.4.3 Shearing strength for ships with two effective longitudinal bulkheads**

The thickness of side shell and longitudinal bulkheads are not to be less than the values given by the following formulae:

For side shell:

\[
t = \left[ (0.5 - \phi) (F_s + F_w) + \Delta F_{sh} \right] \frac{S}{I} \times 10^2 \quad \text{(mm)}
\]

For longitudinal bulkheads:

\[
t = \left[ \phi (F_s + F_w) + \Delta F_{bl} \right] \frac{S}{I} \times 10^2 \quad \text{(mm)}
\]

where, $\phi$ = ratio of shear force shared by the longitudinal bulkhead to the total shear force, and given by each Classification Society.
\(\Delta F_{sh}, \Delta F_{bl}\) = shear force acting upon the side shell plating and longitudinal bulkhead plating, respectively, due to local loads, and given by each Classification Society, subject to the sign convention specified in S11.2.1.1

\(S, I, \tau\) = As specified in S11.4.2 (i)
S 11.5 Buckling strength

S 11.5.1 Application

These requirements apply to plate panels and longitudinals subject to hull girder bending and shear stresses.

S 11.5.2 Elastic buckling stresses

S 11.5.2.1 Elastic buckling of plates

1. Compression

The ideal elastic buckling stress is given by:

\[ \sigma_E = 0.9mE \left( \frac{t_b}{1000s} \right)^2 \text{ (N/mm}^2\text{)} \]

For plating with longitudinal stiffeners (parallel to compressive stress):

\[ m = \frac{8.4}{\Psi + 1.1} \quad \text{for} \quad (0 \leq \Psi \leq 1) \]

For plating with transverse stiffeners (perpendicular to compressive stress):

\[ m = c \left[ 1 + \left( \frac{s}{\ell} \right)^2 \right] \left( \frac{2.1}{\Psi + 1.1} \right) \quad \text{for} \quad (0 \leq \Psi \leq 1) \]

where

- \( E \) = modulus of elasticity of material
  - \( = 2.06 \times 10^5 \text{ N/mm}^2 \) for steel

- \( t_b \) = net thickness, in mm, of plating, considering standard deductions equal to the values given in the table here after:
<table>
<thead>
<tr>
<th>Structure</th>
<th>Standard deduction (mm)</th>
<th>Limit values min-max (mm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>- Compartments carrying dry bulk cargoes</td>
<td>0.05 t</td>
<td>0.5 - 1</td>
</tr>
<tr>
<td>- One side exposure to ballast and/or liquid cargo</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Vertical surfaces and surfaces sloped at an angle greater than 25° to the horizontal line</td>
<td></td>
<td></td>
</tr>
<tr>
<td>- One side exposure to ballast and/or liquid cargo</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Horizontal surfaces and surfaces sloped at an angle less than 25° to the horizontal line</td>
<td></td>
<td></td>
</tr>
<tr>
<td>- Two side exposure to ballast and/or liquid cargo</td>
<td>0.10 t</td>
<td>2 - 3</td>
</tr>
<tr>
<td>Vertical surfaces and surfaces sloped at an angle greater than 25° to the horizontal line</td>
<td></td>
<td></td>
</tr>
<tr>
<td>- Two side exposure to ballast and/or liquid cargo</td>
<td>0.15 t</td>
<td>2 - 4</td>
</tr>
<tr>
<td>Horizontal surfaces and surfaces sloped at an angle less than 25° to the horizontal line</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>
s = shorter side of plate panel, in m, 
ℓ = longer side of plate panel, in m, 
c = 1.3 when plating stiffened by floors or deep girders, 
   = 1.21 when stiffeners are angles or T-sections, 
   = 1.10 when stiffeners are bulb flats, 
   = 1.05 when stiffeners are flat bars, 
Ψ = ratio between smallest and largest compressive σ stress when linear variation across panel.

2. Shear
The ideal elastic buckling stress is given by:

\[ \tau_E = 0.9k_t E \left( \frac{t_b}{1000s} \right)^2 \quad \text{(N/mm}^2\text{)} \]

\[ K_t = 5.34 + 4 \left( \frac{s}{\ell} \right)^2 \]

E, t_b, s and ℓ are given in 1.

S11.5.2.2 Elastic buckling of longitudinals

1. Column buckling without rotation of the cross section
For the column buckling mode (perpendicular to plane of plating) the ideal elastic buckling stress is given by:

\[ \sigma_E = 0.001E \frac{I_a}{A\ell^2} \quad \text{(N/mm}^2\text{)} \]

\[ I_a = \text{moment of inertia, in cm}^4, \text{ of longitudinal, including plate flange and calculated with thickness as specified in S 11.5.2.1.1.} \]

\[ A = \text{cross-sectional area, in cm}^2, \text{ of longitudinal, including plate flange and calculated with thickness as specified in S 11.5.2.1.1,} \]

\[ \ell = \text{span, in m, of longitudinal,} \]

A plate flange equal to the frame spacing may be included.

2. Torsional buckling mode
The ideal elastic buckling stress for the torsional mode is given by:

\[ \sigma_E = \frac{\pi^2 E L_w}{10^4 I_r \ell^2} \left( m^2 + \frac{K}{m^2} \right) + 0.385 E \frac{I_a}{I_p} \quad \text{(N/mm}^2\text{)} \]

\[ K = \frac{C\ell^4}{\pi^4 E I_w} 10^6 \]
$m = \text{number of half waves, given by the following table:}$

<table>
<thead>
<tr>
<th>$0 &lt; K &lt; 4$</th>
<th>$4 &lt; K &lt; 36$</th>
<th>$36 &lt; K &lt; 144$</th>
<th>$(m-1)^2 &lt; K \leq m^2 (m+1)^2$</th>
</tr>
</thead>
<tbody>
<tr>
<td>$m$</td>
<td>$1$</td>
<td>$2$</td>
<td>$3$</td>
</tr>
</tbody>
</table>

$I_i = \text{St Venant’s moment of inertia, in cm}^4, \text{ of profile (without plate flange)}$

\[ I_i = \frac{h_w t_w^3}{3} \times 10^{-4} \quad \text{for flat bars (slabs)} \]

\[ I_i = \frac{1}{3} h_w t_w^3 + b_f t_f^3 \left( 1 - 0.63 \frac{t_w}{b_f} \right) \times 10^{-4} \quad \text{for flanged profiles} \]

$I_p = \text{polar moment of inertia, in cm}^4, \text{ of profile about connection of stiffener to plate}$

\[ I_p = \frac{h_w^3 t_w}{3} \times 10^{-4} \quad \text{for flat bars (slabs)} \]

\[ I_p = \left( \frac{h_w^3 t_w}{3} + h_w^2 b_f t_f \right) \times 10^{-4} \quad \text{for flanged profiles} \]

$I_w = \text{sectorial moment of inertia, in cm}^6, \text{ of profile about connection of stiffener to plate}$

\[ I_w = \frac{h_w^3 t_w}{36} \times 10^{-6} \quad \text{for flat bars (slabs)} \]

\[ I_w = \frac{t_f b_f^2 h_w^2}{12} \times 10^{-6} \quad \text{for "Tee" profiles} \]

\[ I_w = \frac{b_f^2 h_w^2}{12(b_f + h_w)^2} \left[ t_f (b_f^2 + 2b_f h_w + 4h_w^2) + 3t_w b_f h_w \right] \times 10^{-6} \quad \text{for angles and bulb profiles} \]

$h_w = \text{web height, in mm,}$

$t_w = \text{web thickness, in mm, considering standard deductions as specified in S 11.5.2.1.1,}$

$b_f = \text{flange width, in mm,}$

$t_f = \text{flange thickness, in mm, considering standard deductions as specified in S 11.5.2.1.1. For bulb profiles the mean thickness of the bulb may be used.}$
\( \ell \) = span of profile, in m,
\( s \) = spacing of profiles, in m.

\[ C = \text{spring stiffness exerted by suporting plate } p \]
\[ = \frac{k_p E t_p^3}{3s \left( 1 + \frac{1.33k_p h_w t_p^3}{1000st_w^3} \right) 10^{-3}} \]

\( k_p = 1 - \eta_p \) not to be taken less than zero
\( t_p = \) plate thickness, in mm, considering standard deductions as specified in S 11.5.2.1.1.
\( \eta_p = \frac{\sigma_a}{\sigma_{Ep}} \)
\( \sigma_a = \) calculated compressive stress. For longitudinals, see S 11.5.4.1,
\( \sigma_{Ep} = \) elastic buckling stress of supporting plate as calculated in S 11.5.2.1,
For flanged profiles, \( k_p \) need not be taken less than 0.1.

3. Web and flange buckling

For web plate of longitudinals the ideal elastic buckling stress is given by:
\[ \sigma_E = 3.8E \left( \frac{t_w}{h_w} \right)^2 \text{ (N/mm}^2 \text{)} \]

For flanges on angles and T-sections of longitudinals, buckling is taken care of by the following requirement:
\[ \frac{b_f}{t_f} \leq 15 \]

\( b_f = \) flange width, in mm, for angles, half the flange width for T-sections.
\( t_f = \) as built flange thickness.

**S 11.5.3 Critical buckling stresses**

**S 11.5.3.1 Compression**

The critical buckling stress in compression \( \sigma_c \) is determined as follows:
\( \sigma_c = \sigma_E \quad \text{when} \quad \sigma_E \leq \frac{\sigma_F}{2} \)

\[ = \sigma_F \left( 1 - \frac{\sigma_E}{4\sigma_F} \right) \quad \text{when} \quad \sigma_E > \frac{\sigma_F}{2} \]

\( \sigma_F \) = yield stress of material, in N/mm\(^2\); \( \sigma_F \) may be taken as 235 N/mm\(^2\) for mild steel,

\( \sigma_E \) = ideal elastic buckling stress calculated according to S 11.5.2.

**S 11.5.3.2 Shear**

The critical buckling stress in shear \( \tau_c \) is determined as follows:

\[ \tau_c = \tau_E \quad \text{when} \quad \tau_E \leq \frac{\tau_F}{2} \]

\[ = \tau_F \left( 1 - \frac{\tau_E}{4\tau_F} \right) \quad \text{when} \quad \tau_E > \frac{\tau_F}{2} \]

\( \tau_F = \frac{\sigma_F}{\sqrt{3}} \)

\( \sigma_F \) = as given in S 11.5.3.1,

\( \tau_E \) = ideal elastic buckling stress in shear calculated according to S11.5.2.1.2.

**S 11.5.4 Working stress**

**S 11.5.4.1 Longitudinal compressive stresses**

The compressive stresses are given in the following formula:

\[ \sigma_a = \frac{M_s + M_w}{I_n} y \cdot 10^5 \text{ N/mm}^2 \]

\[ = \text{minimum} \frac{30}{k} \text{ N/mm}^2 \]

\( M_s \) = still water bending moment (kN.m), as given in S 11.2.1,

\( M_w \) = wave bending moment (kN.m) as given in S 11.2.2.1,

\( I_n \) = moment of inertia, in cm\(^4\), of the hull girder,

\( y \) = vertical distance, in m, from neutral axis to considered point.

\( k \) = as specified in S 11.3.1.1 (i).

\( M_s \) and \( M_w \) are to be taken as sagging or hogging bending moments, respectively, for members above or below the neutral axis.

Where the ship is always in hogging condition in still water, the sagging bending moment \((M_s + M_w)\) is to be specially considered.
S 11.5.4.2 \textit{Shear stresses}

1. Ships without effective longitudinal bulkheads
   For side shell
   \[ \tau_a = \frac{0.5 |F_s + F_w|}{t} \cdot \frac{S}{I} \cdot 10^3 \text{ N/mm}^2 \]
   \(F_s, F_w, t, s, I\) as specified in S 11.4.2

2. Ships with two effective longitudinal bulkheads
   For side shell
   \[ \tau_a = \frac{|(0.5 - \phi)(F_s + F_w) + \Delta F_{sh}|}{t} \cdot \frac{S}{I} \cdot 10^3 \text{ N/mm}^2 \]
   For longitudinal bulkheads
   \[ \tau_a = \frac{|\phi (F_s + F_w) + \Delta F_{bl}|}{t} \cdot \frac{S}{I} \cdot 10^3 \text{ N/mm}^2 \]
   \(F_s, F_w, \Delta F_{sh}, \Delta F_{bl}, t, s, I\) as specified in S 11.4.3.

S 11.5.5 Scantling criteria

S 11.5.5.1 \textit{Buckling Stress}

The design buckling stress \(\sigma_c\) of plate panels and longitudinals (as calculated in S 11.5.3.1) is not to be less than:

\[ \sigma_c \geq \beta \sigma_a \]

where

\[ \beta = 1 \quad \text{for plating and for web plating of stiffeners (local buckling)} \]

\[ \beta = 1.1 \quad \text{for stiffeners} \]

The critical buckling stress \(\tau_c\) of plate panels (as calculated in S 11.5.3.2) is not to be less than:

\[ \tau_c \geq \tau_a \]